

Hydraulic load interdependencies: a case study on a regional flood defense system in the Netherlands

Stephan Rikkert, Matthijs Kok

s.j.h.rikkert@tudelft.nl

stowa


TU Delft

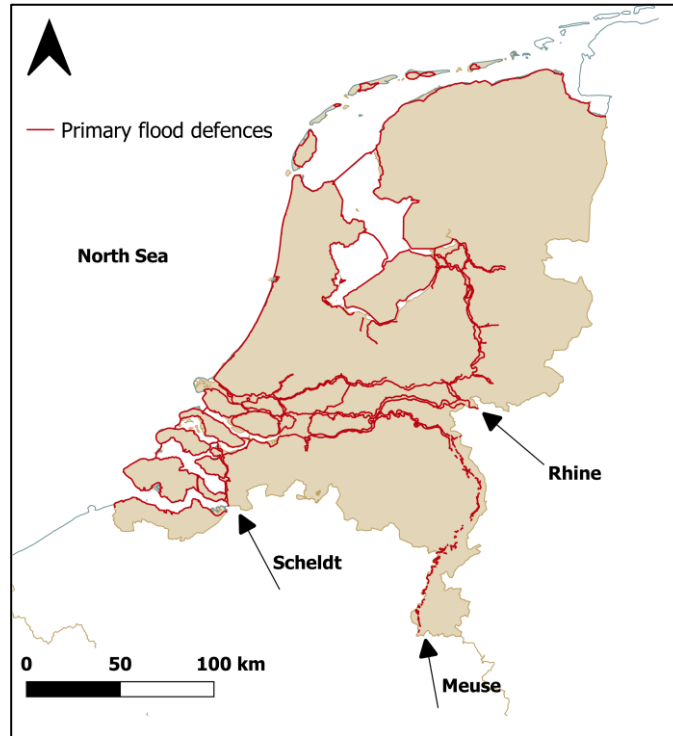
System Risk Conference, Potsdam 2019

Content

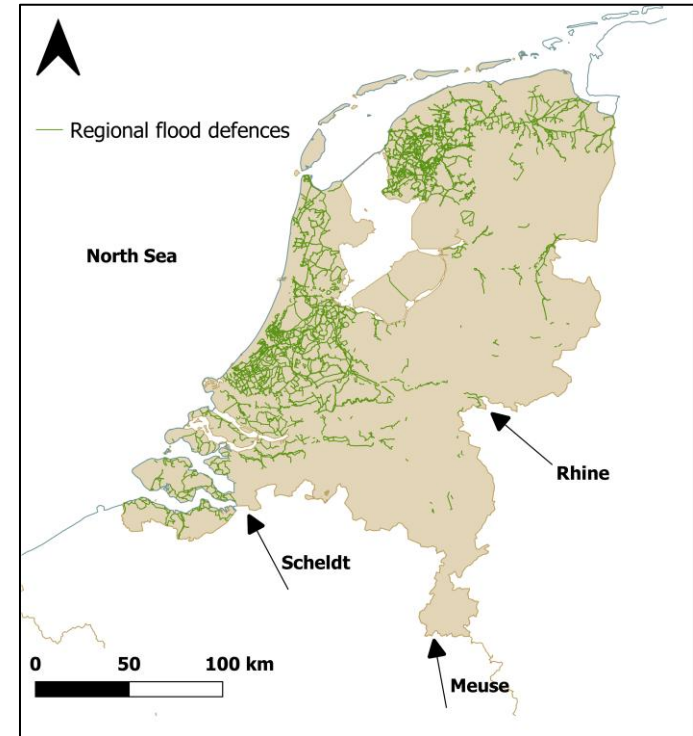
- Introduction
- Method
- Case studies
- Conclusions

Flood defences in the Netherlands

±3.400 km: primary



±11.500 km: regional



Polder canals

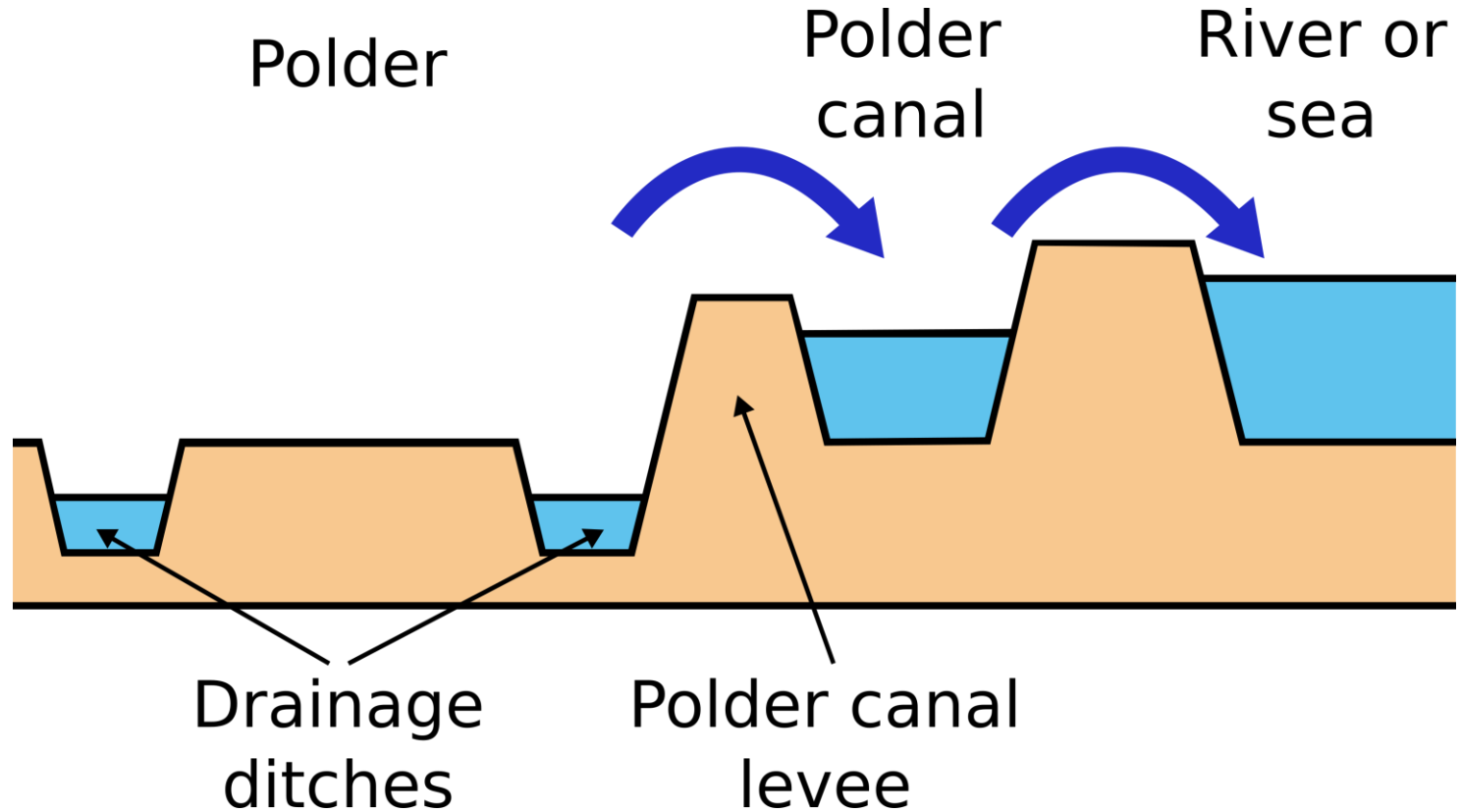
Approximately 80% of regional flood defences consists of polder canal levees (in Dutch: boezemkaden): >8,000 km

(<https://beeldbank.rws.nl>,
Rijkswaterstaat /
Rob van der Laag)



Introduction
Method
Case studies
Conclusions

Polder canals



Introduction
Method
Case studies
Conclusions

Levee failures

Tuindorp Oostzaan, 1960

Cause: unknown, most likely ruptured water pipe



Wilnis, 2003

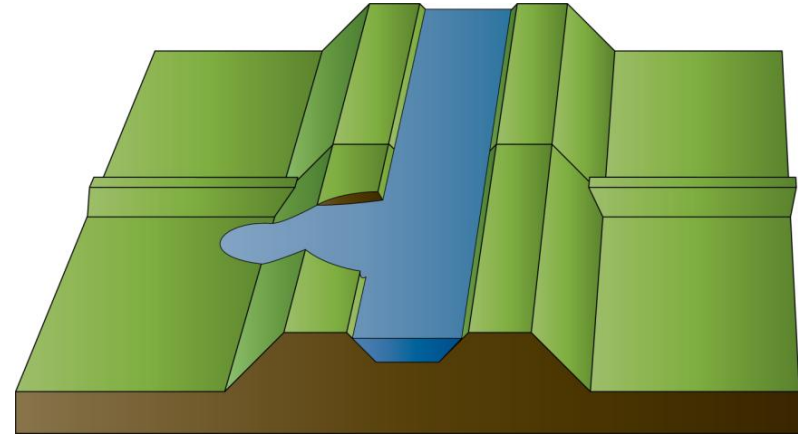
Cause: drying of a peat dike

Description

Schematic representation

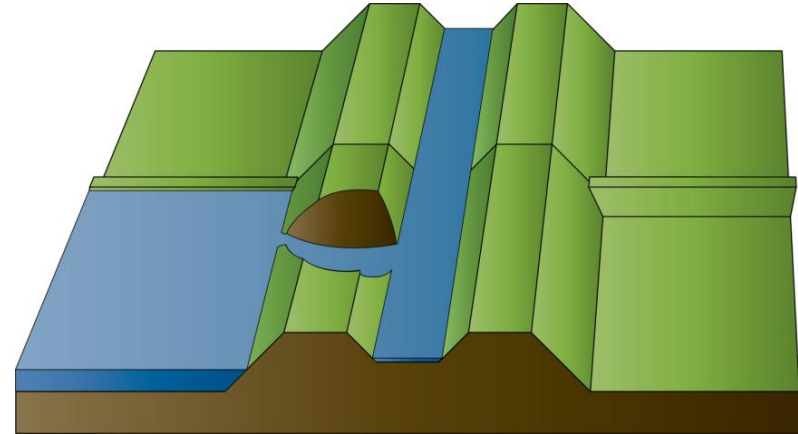
Phase 1

- Hydraulic load larger than levee resistance;
- Breach starts growing



Phase 2

- Breach has grown;
- Adjacent polder fills with water;
- Canal water level drops;
- 'Surviving' levees 'relieved'.



Objective

Develop a method to include hydraulic load interdependency in flood safety assessments for regional flood defense systems

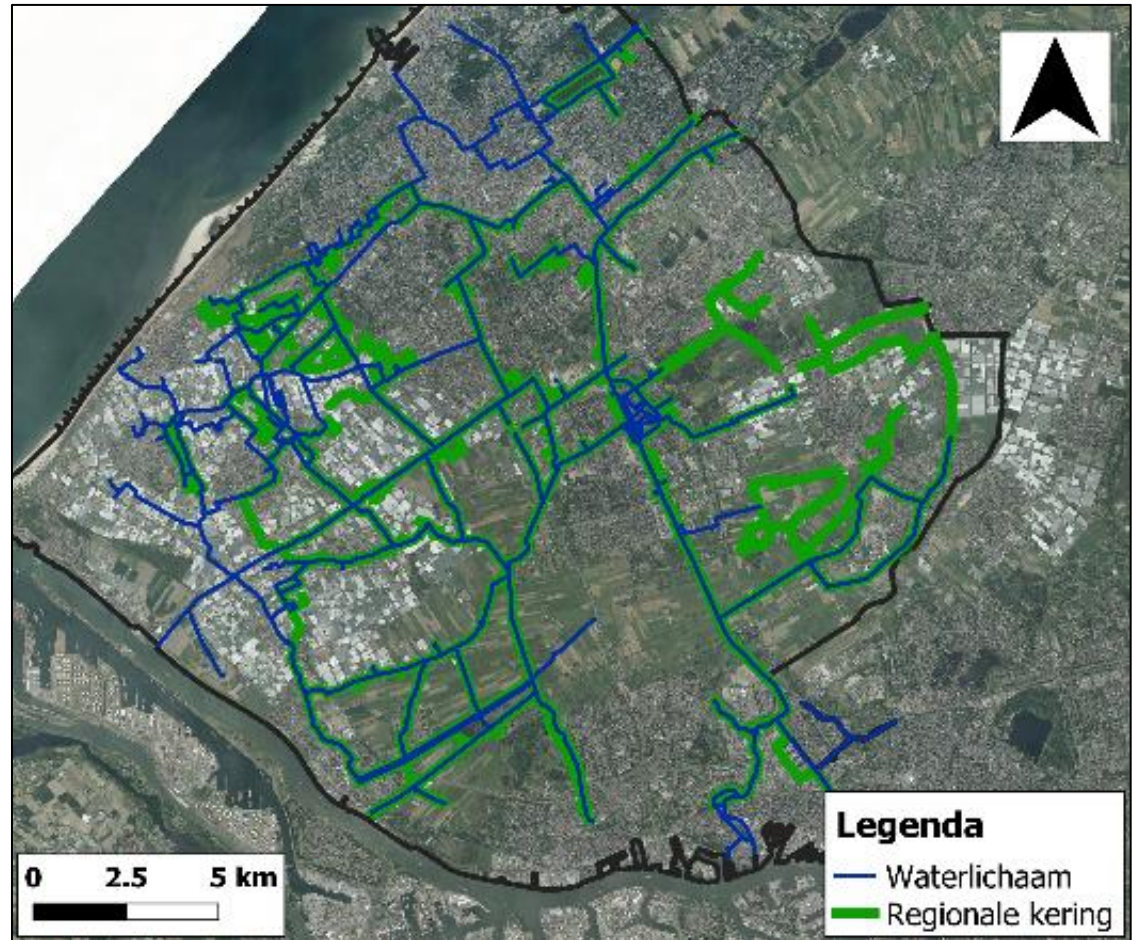
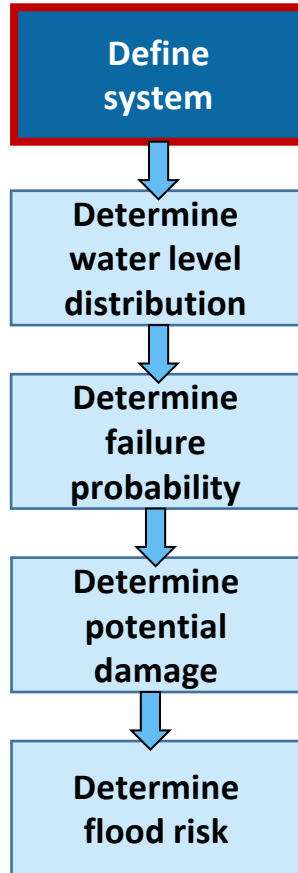
Introduction

Method

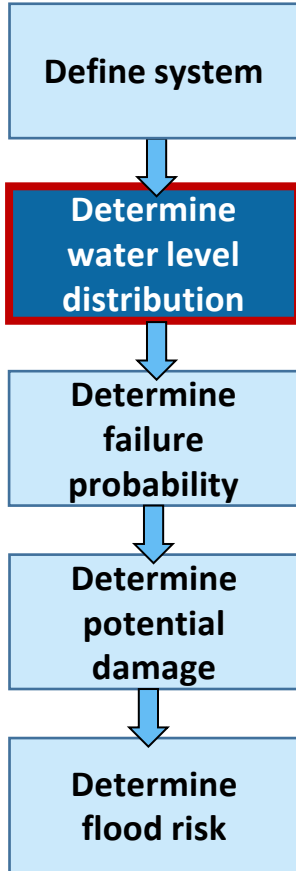
Case studies

Conclusions

Method



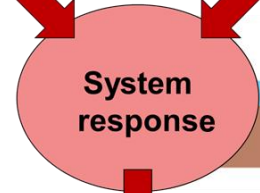
Method



Rainfall
(and wind)

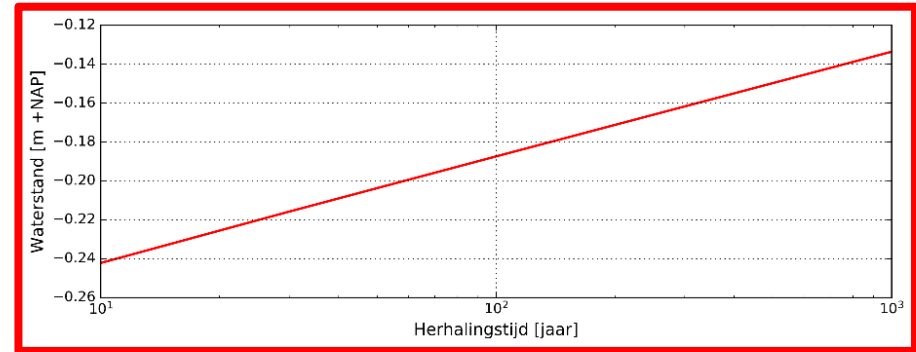
Initial conditions
system

Available storage
capacity



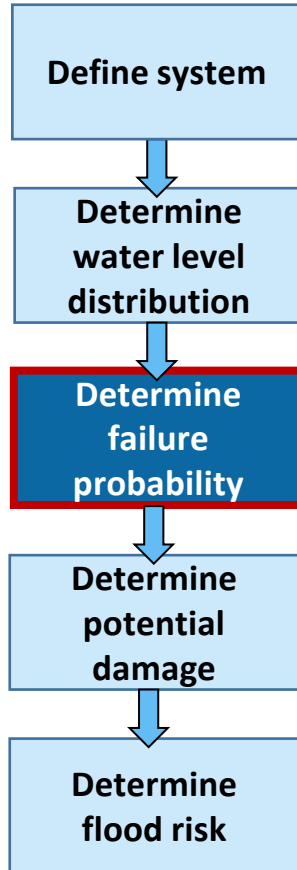
Water level

Exceedance frequency curve

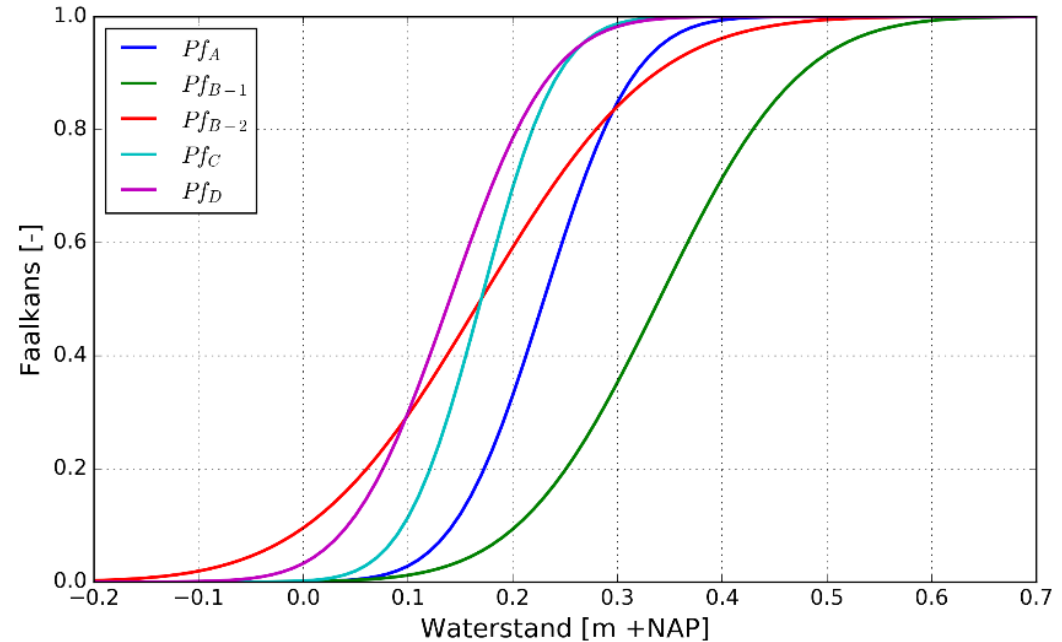


Introduction
Method
Case studies
Conclusions

Method

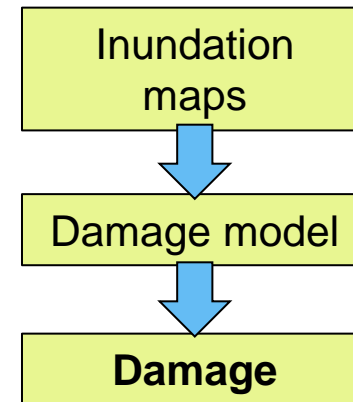
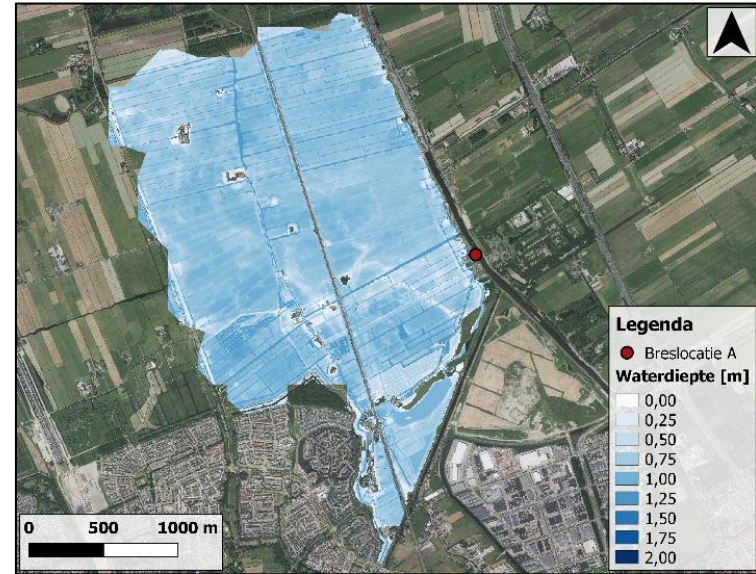
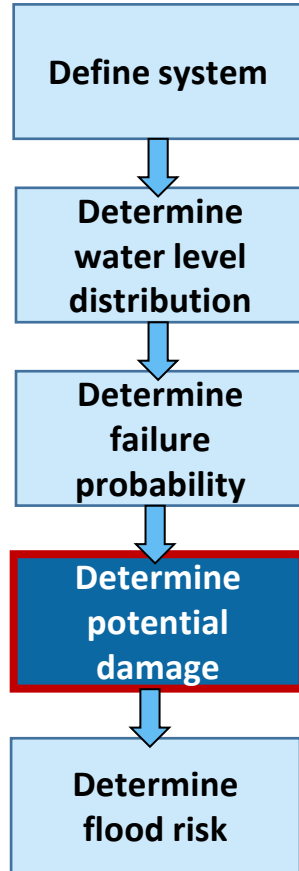


Fragility curves

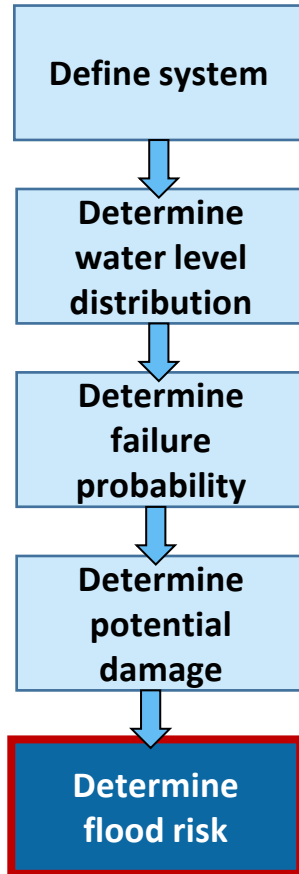


Failure probability P_f as a function of the water level

Method

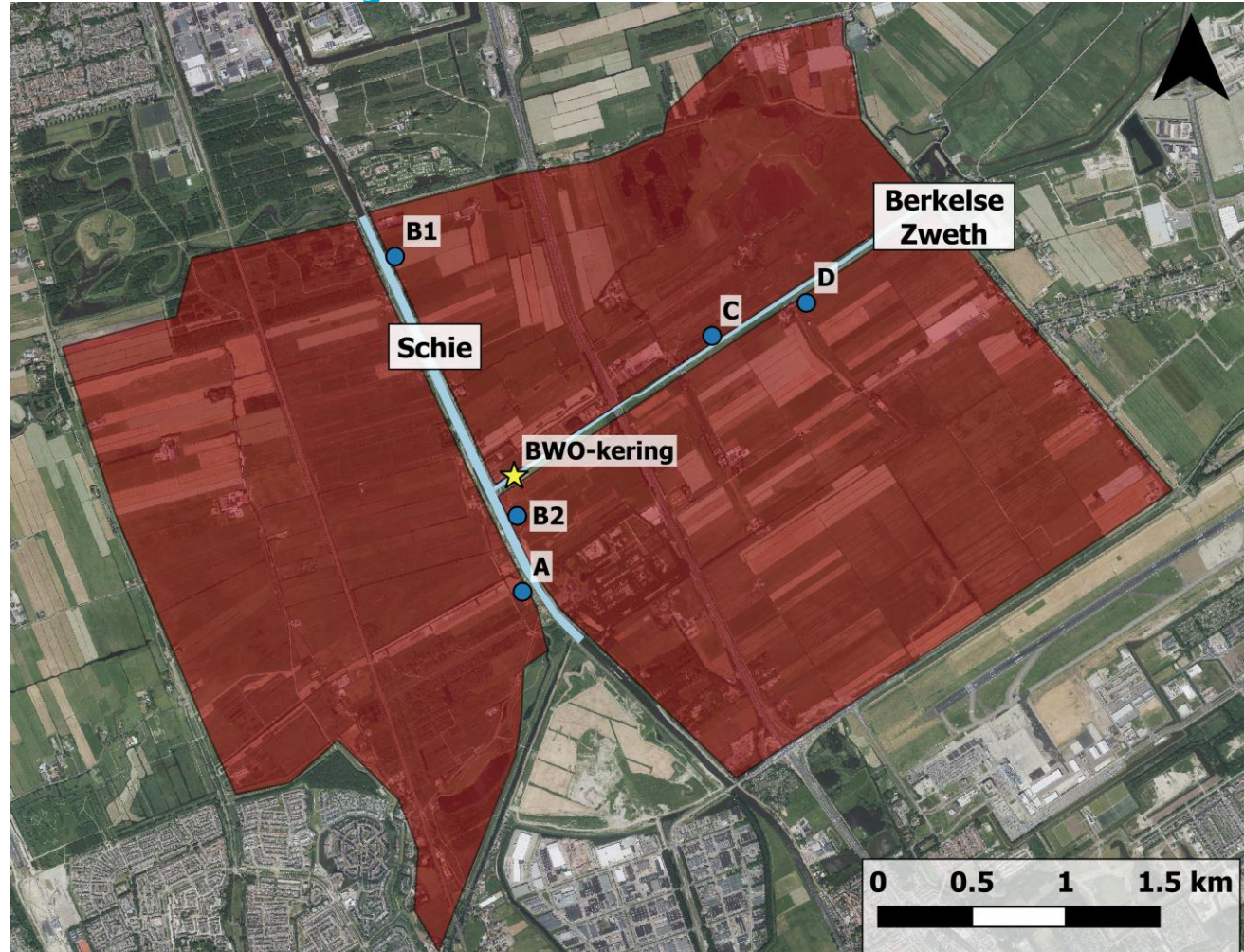


Method



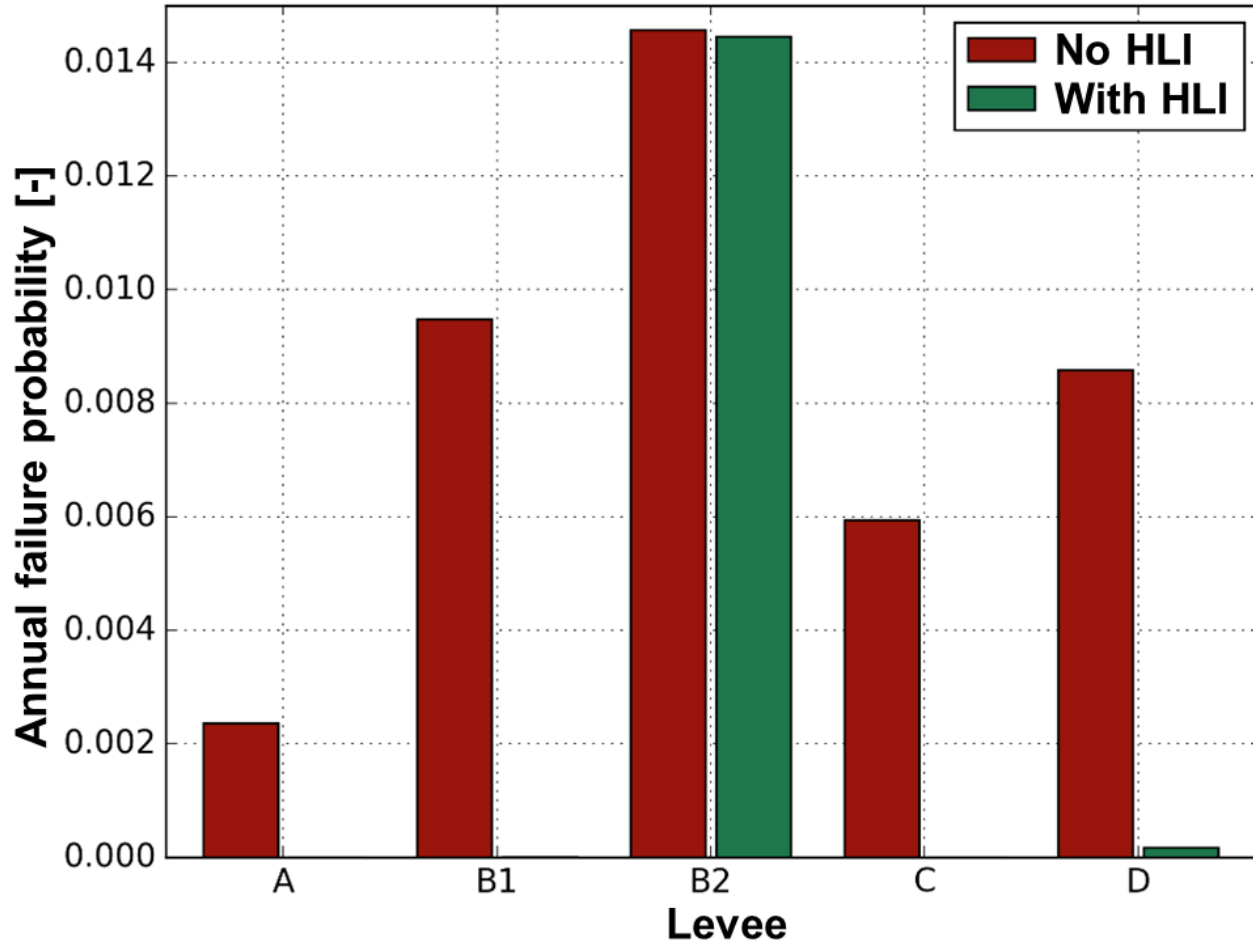
Risk = probability x consequences

Case study Delfland



Introduction
Method
Case studies
Conclusions

Case study Delfland

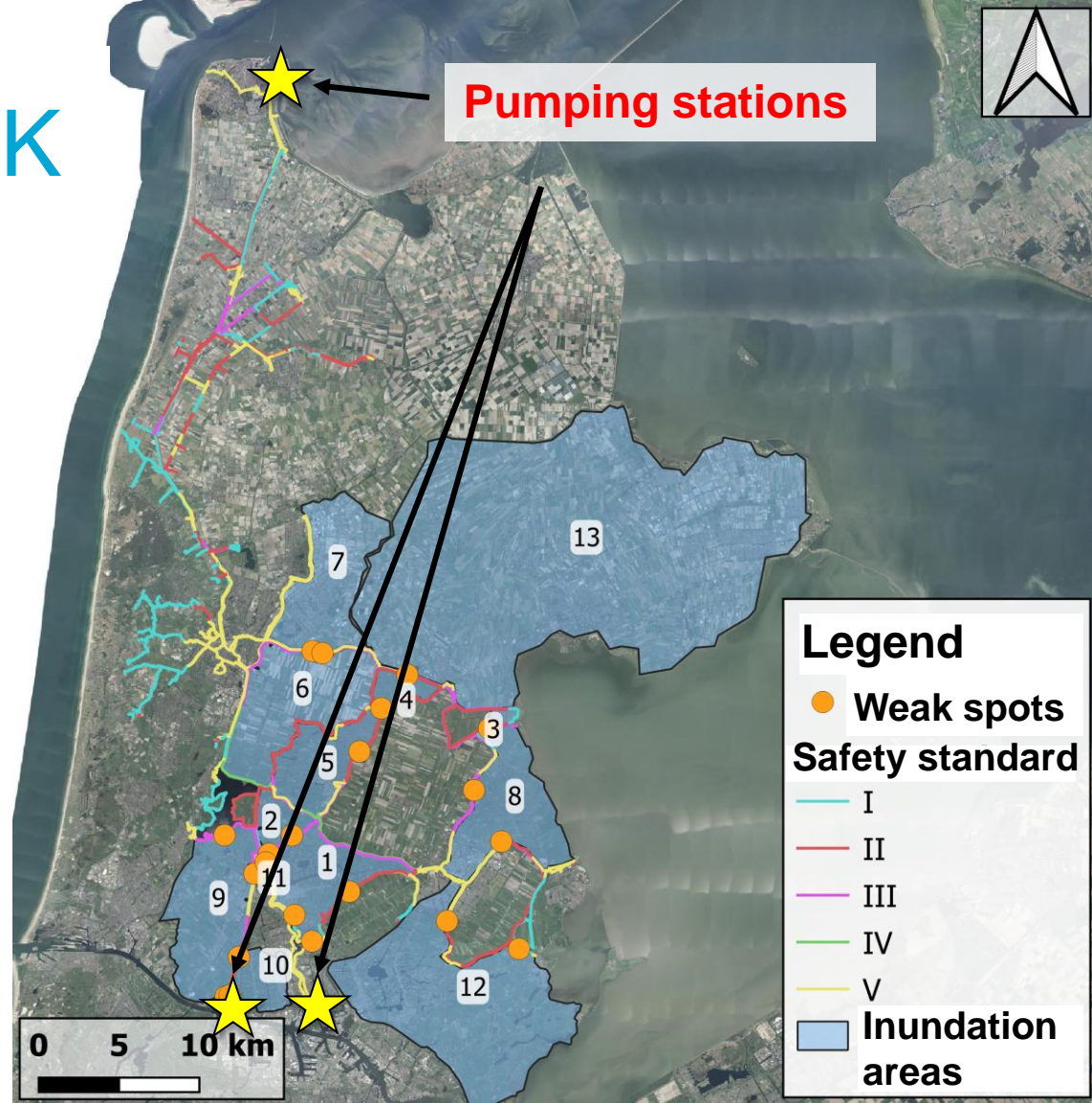


Case study HHNK

Water board Hollands
Noorderkwartier

Schermerboezem

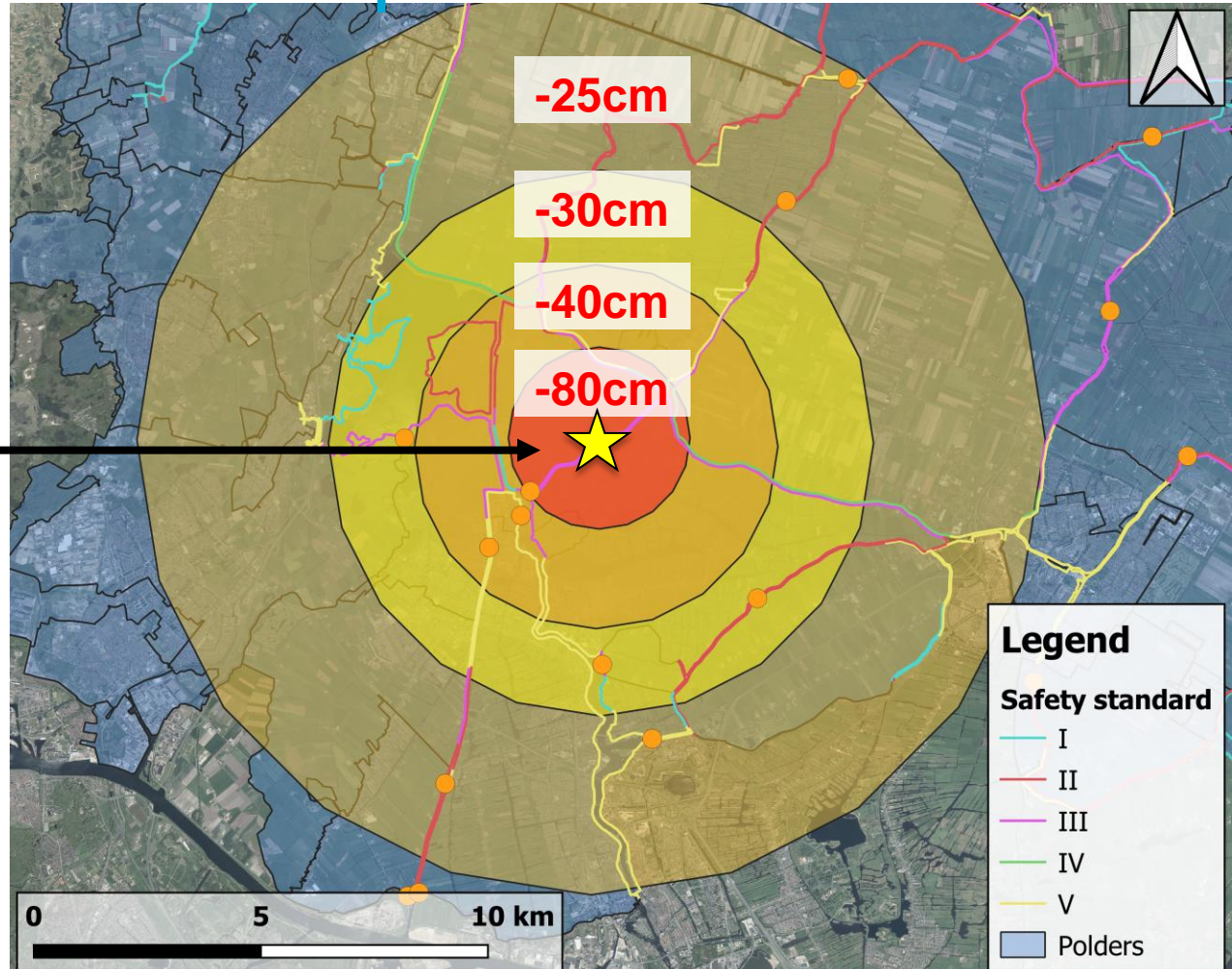
> 660 km of levees



Water level drop

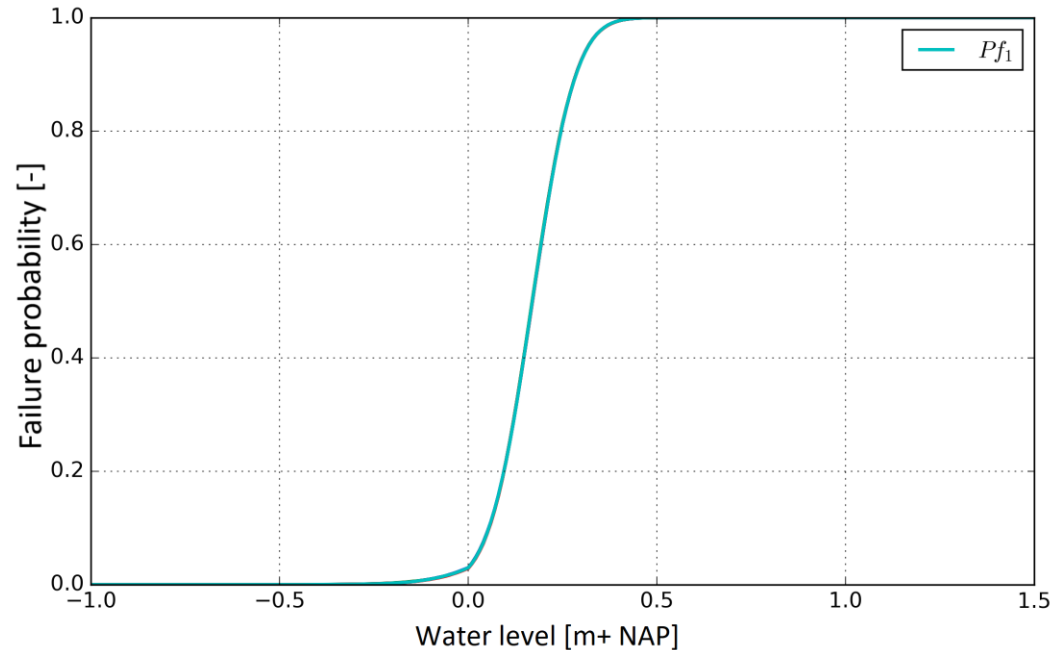
Introduction
Method
Case studies
Conclusions

**Levee
breach**



Fragility curves

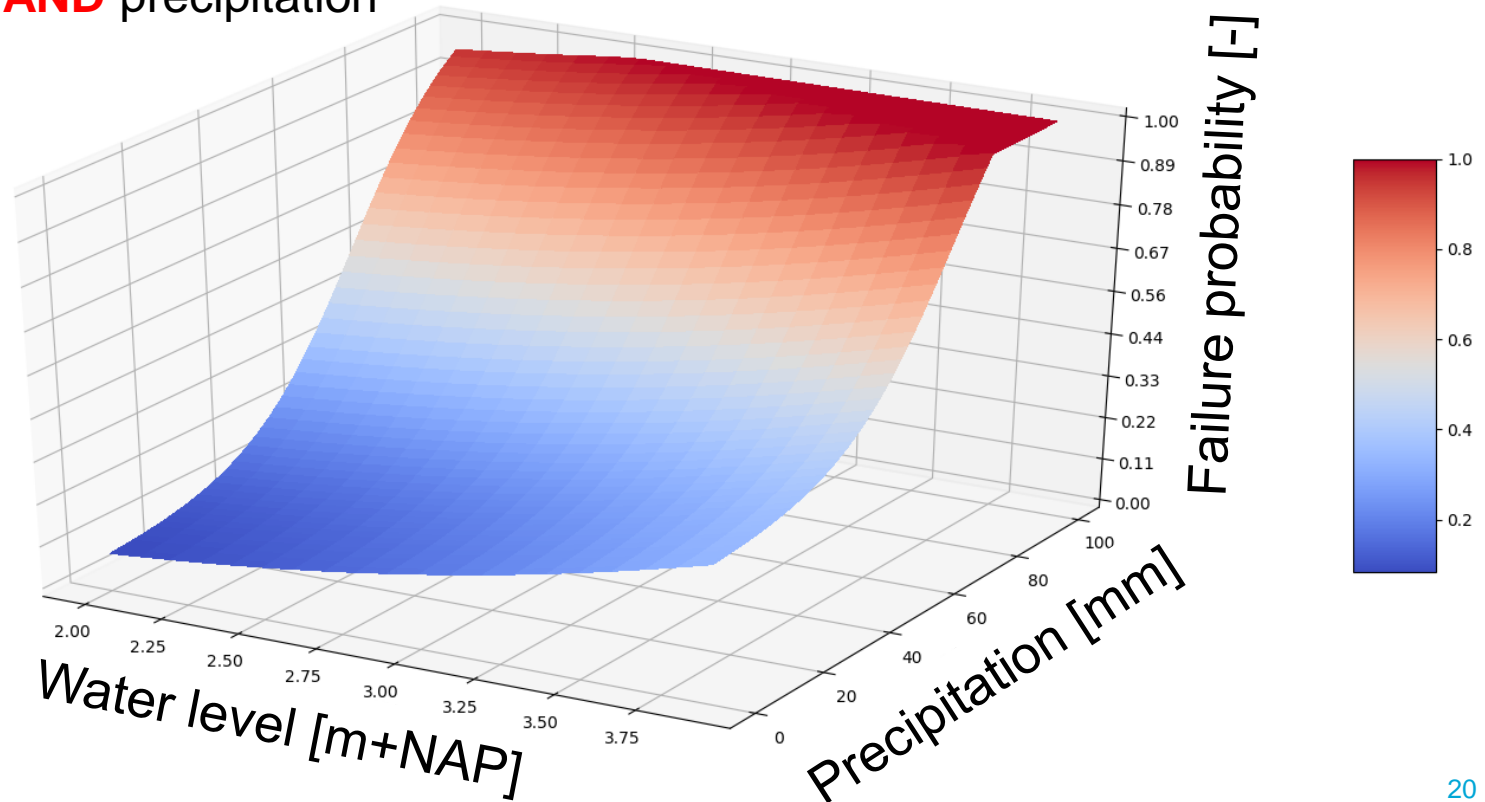
Failure probability as a function of the water level



BUT... water level hardly varies!

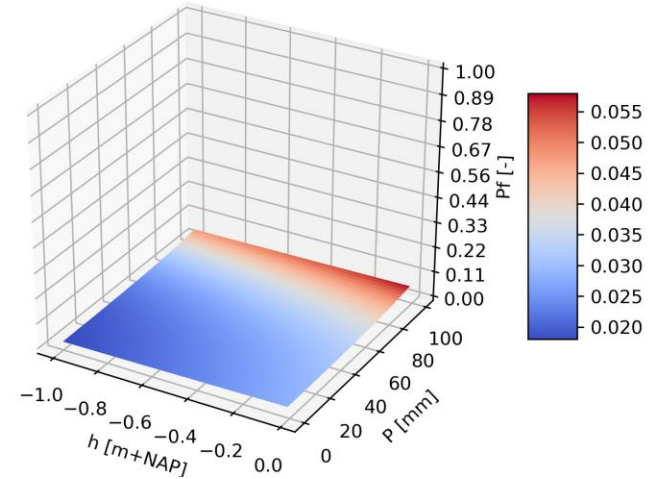
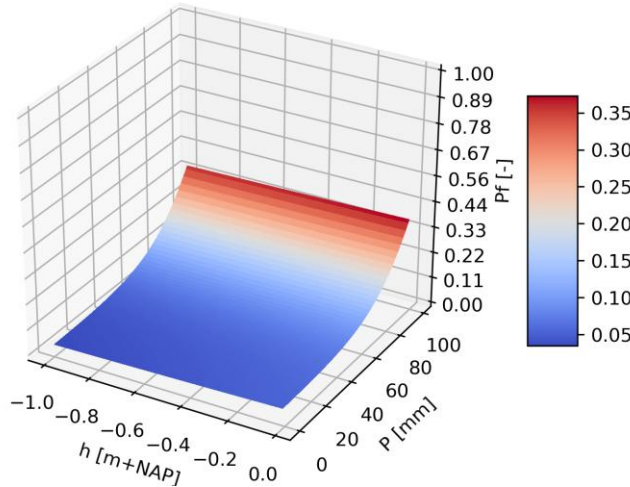
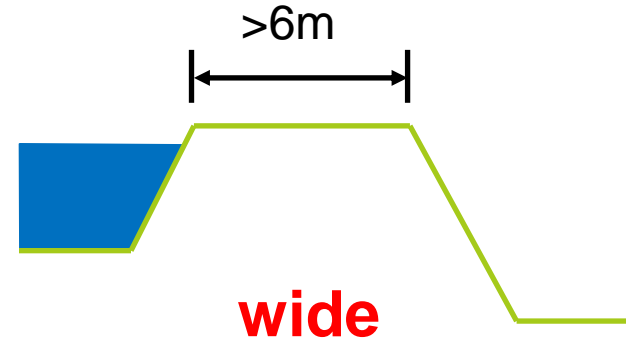
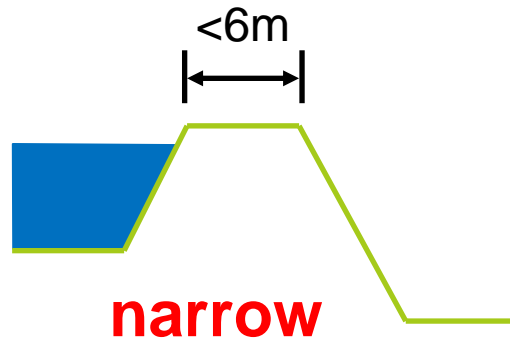
Fragility surface

Failure probability as a function of **BOTH** the water level **AND** precipitation



Introduction
Method
Case studies
Conclusions

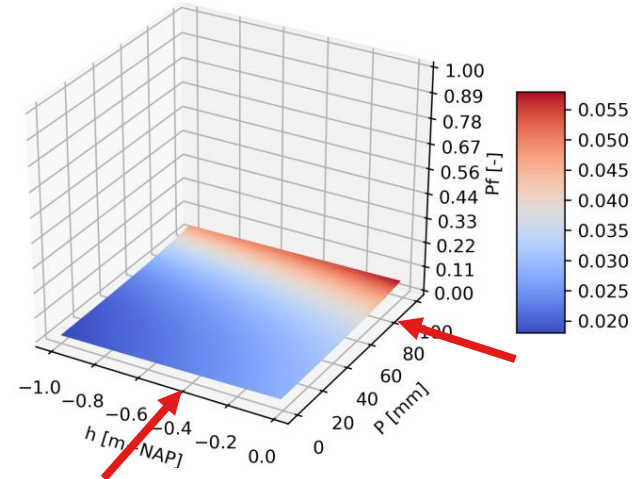
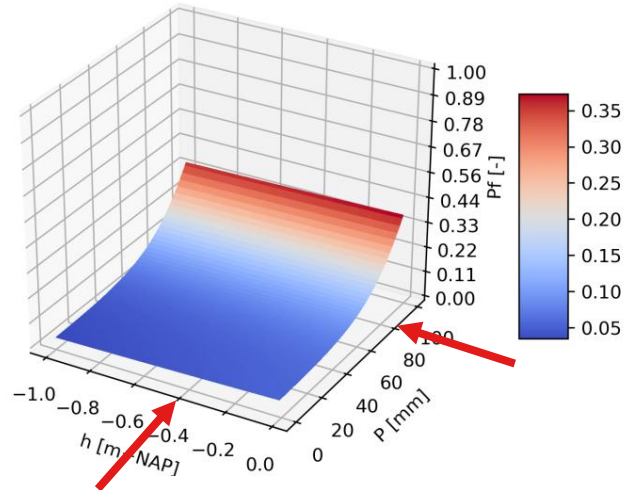
Fragility surface



Results

Event:

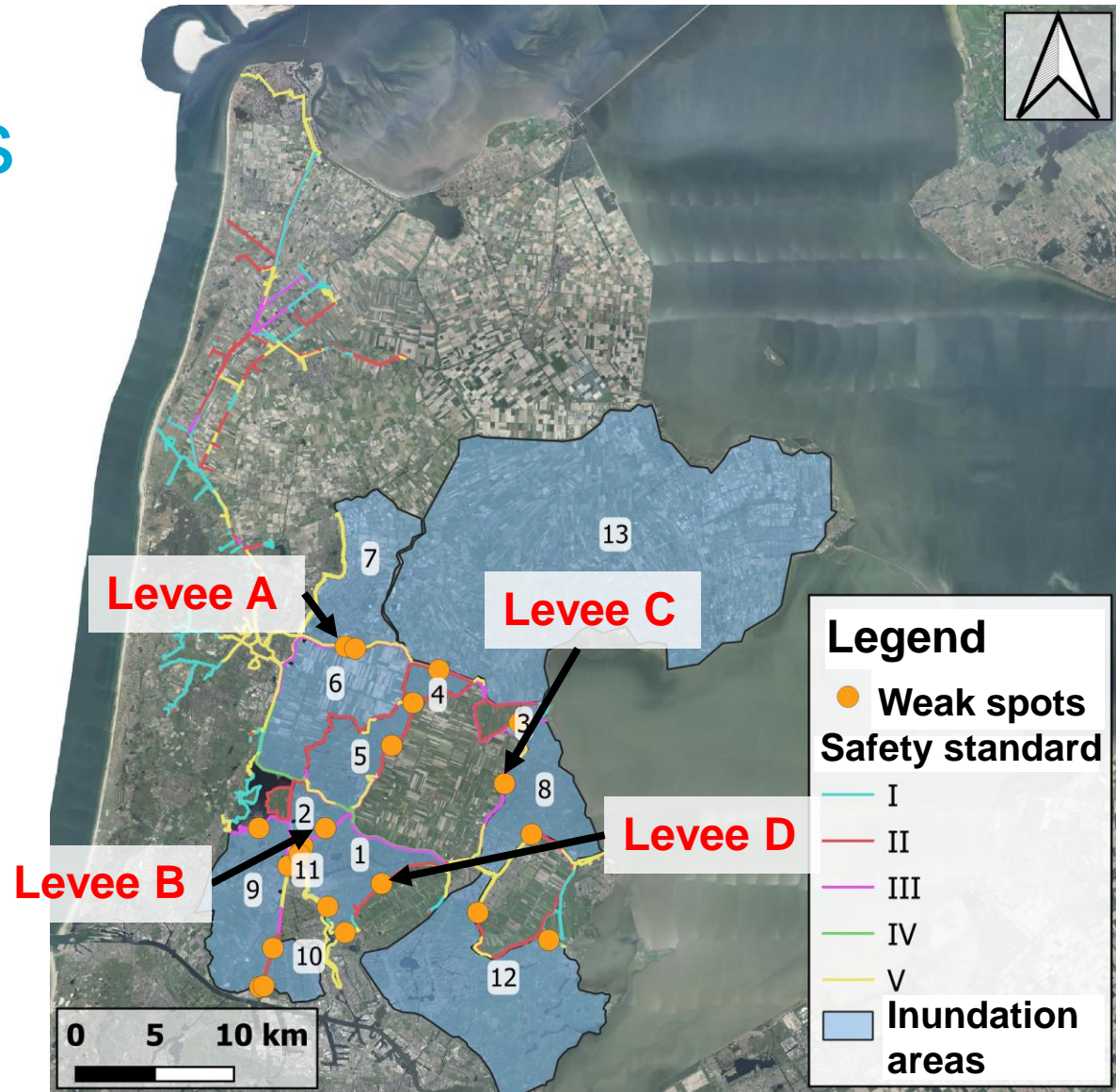
- Water level: $-0.40 \text{ m} + \text{NAP}$
- Precipitation: 80 mm in 24 hrs



	Narrow dikes	Wide dikes
Conditional fail. prob. [-]	0.1	0.033

Results

	Pf [-]	
Levee	No HLI	HLI
A	0.033	0.027
B	0.1	0.056
C	0.1	0.066
D	0.1	0.054



Preliminary conclusions

- Levee breach affects hydraulic loads;
- Breach location matters;
- Not taking into account for hydraulic load interdependency leads to errors in risk assessment.

Hydraulic load interdependencies: a case study on a regional flood defense system in the Netherlands

Stephan Rikkert, Matthijs Kok

s.j.h.rikkert@tudelft.nl

stowa


TU Delft

System Risk Conference, Potsdam 2019